

Comparative Seismic Analysis of Multi Storey RC Building by Considering the Effect of Dual System

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Abstract— Due to the current progress in world population, people in this world tend to occupy available locations present in any zone which also include zones falling in the high seismic zone categories. The structures to be built in these seismic zones are more susceptible to earthquakes for obvious reasons, and the structures constructed in such zones must be analysed and designed for the unpredictable earthquakes with unpredictable magnitudes by various lateral load resisting systems such as shear walls, bracings, tubular systems, coupled shear walls and even a combination of two load resisting systems called as dual systems. Present study includes linear-static and non-linear static analysis with different shear wall arrangements on dual systems such as flat slabs and shear walls & moment resisting frames and shear walls for different irregular plans using ETABS software. Parameters such as point displacements, base shears, pushover curves are studied.

Keywords: Dual Systems, Flat Slabs, Pushover Analysis, Shear Walls

I. INTRODUCTION

At present there has been a gigantic increment in the measure of high stories in current regions and their extraordinary fear is on the presence of the building which should be high slender. Thus it's critical for these buildings to oppose horizontal forces laterally with vertical forces. With this decisions the structure would be dealt with presentation wise, meanwhile the buildings being high and slender are exposed to seismic & wind loads. Dual system has been perceived to fight lateral loads viably, meanwhile it is a combination of two load resisting systems. Flat slab with shear wall and Moment Resisting Frame (MRF) with shear wall can be used as a dual system.

Buildings are usually designed for a minor seismic movement & experience a severe non-linear movement in earthquake defiant design. The performance and ductility characteristics of a structure are obligatory in order to study the structure under breakdown. True dynamic analysis is most of the times not viable and hence in the recent years an analysis called pushover analysis is being used which assesses various parameters such as, base shear, displacement, loads drifts, etc.

The dual system is used to resist lateral loads by merging the two lateral load resisting systems. In this systems the profile of the distortion will be different from those in wall and frame systems, where causing interlaced force take place and modification of the figure of shear and moment diagrams. The advantage of this combination is that the frames support the wall at the topmost & control their displacement and the wall supports the frame at the bottommost and declines their displacement. It delivers an enhanced immunity to the structure subsisting in an earthquake prone zone. A variety

of Dual systems can be used such as special moment resisting frame (SMRF) with Shear walls, SMRF with Bracing, SMRF with Infill wall, Flat Slabs with Shear wall, Bracing and Infill wall each etc.

II. METHODOLOGY

A G+14 storey building measuring 30m x 25m in plan having E- Shape and diaphragm discontinuity is modelled. There are 10 models analysed in the present study by considering SMRF with flat slab and shear wall as dual system. Shear walls are used for E-shaped and diaphragm discontinuity models at different locations. For the present study, nonlinear static analysis and linear static analysis is considered.

A. Description of Building Structure

The details of building is given in Table 1

| STRUCTURE | DESCRIPTION |
|----------------------------|-----------------------|
| Plan Dimension | 30mx25m |
| No of Stories | 15 |
| Height of one storey | 3 m |
| Height of Base Storey | 3.5 m |
| Soil Type | Medium Soil |
| Seismic Zone | V |
| Importance Factor | 1 |
| Grade of Concrete f_{ck} | M30 |
| Grade of Steel f_y | Fe 415 |
| Size of the Beam | 300mmX450mm |
| Size of the Column | 500mmX500mm |
| Slab Thickness | 150 mm |
| Live Load | 3 kN/m ² |
| Live Load on Roof | 2.5 kN/m ² |
| Floor Finish | 1 kN/m ² |
| Column Drop | 300mm |
| Shear Wall Thickness | 200mm |

Table 1:

III. MODELING AND ANALYSIS

Analytical study of equivalent static analysis and push over analysis to obtain various results like base shear, storey shear, storey drift & point displacement are considered. The results obtained after analysing by both linear and non-linear static analysis are compared for model 1 to model 10.

E-Shaped Bare Frame model. (M1)

E- Shaped model with SMRF and shear wall at re-entrant corners. (M2)

E- Shaped model with SMRF and shear wall at Alternate Periphery. (M3)

E- Shaped model with Flat Slab and shear wall at re-entrant corners. (M4)

- E- Shaped model with Flat Slab and shear wall at Alternate Periphery. (M5)
- Diaphragm discontinuity bare frame model (M6)
- Diaphragm discontinuity model with SMRF and shear wall at corners (M7)
- Diaphragm discontinuity model with SMRF and shear wall at alternate Periphery (M8)
- Diaphragm discontinuity model with Flat Slab and shear wall at corners (M9)
- Diaphragm discontinuity model with Flat Slab and shear wall at alternate Periphery (M10)

IV. RESULTS

A. Base Shear

| M1 (kN) | M2 (kN) | M3 (kN) | M4 (kN) | M5 (kN) |
|------------|------------|------------|------------|------------|
| 4805.26 | 5194.11 | 5323.72 | 4880.52 | 5038.8 |

Table 2: Base Shears for "E-Shaped" model

| M6 (kN) | M7 (kN) | M8 (kN) | M9 (kN) | M10 (kN) |
|------------|------------|------------|------------|-------------|
| 5256.46 | 5688.92 | 5904.54 | 5499.18 | 5657.47 |

Table 3: Base Shears for "Diaphragm Discontinuity" model

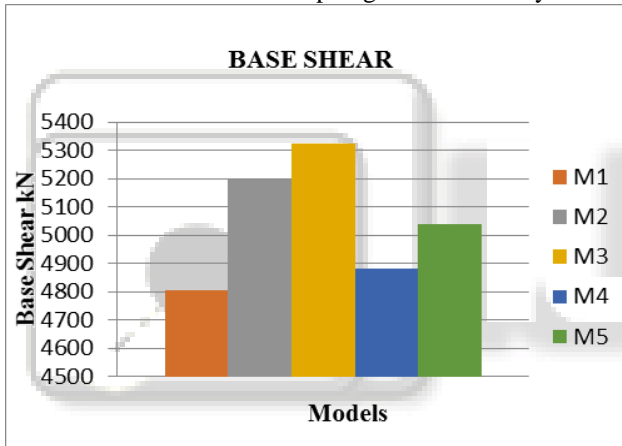


Fig. 11: Base Shear for Model 1 to Model 5

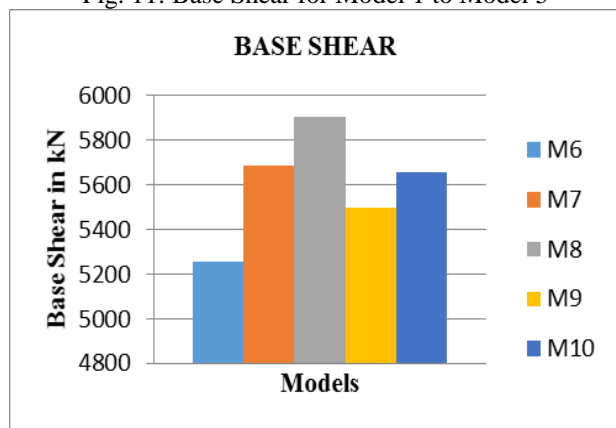


Fig. 12: Base Shear for Model 1 to Model 5

B. Storey Shear

| STORE Y NO | M1 (kN) | M2 (kN) | M3 (kN) | M4 (kN) | M5 (kN) |
|------------|---------|---------|---------|---------|---------|
| 15 | 796.87 | 829.8 | 840.71 | 776.16 | 792.22 |

| | | | | | |
|----|-------------|-------------|-------------|-------------|-------------|
| 14 | 1562.7 3 | 1663.6 5 | 1697.2 4 | 1560.3 4 | 1603.5 7 |
| 13 | 2224.2 8 | 2383.9 4 | 2437.1 1 | 2237.7 3 | 2304.4 3 |
| 12 | 2789.1 6 | 2998.9 7 | 3068.8 7 | 2816.1 2 | 2902.8 7 |
| 11 | 3265 5 | 3517.0 5 | 3601.0 5 | 3303.3 5 | 3406.9 8 |
| 10 | 3659.4 3 | 3946.5 8 | 4042.1 8 | 3707.2 2 | 3824.8 4 |
| 9 | 3980.0 8 | 4295.6 3 | 4400.7 9 | 4035.5 5 | 4164.5 4 |
| 8 | 4234.5 9 | 4572.7 3 | 4685.4 4 | 4296.1 5 | 4434.1 7 |
| 7 | 4430.5 8 | 4786.1 3 | 4904.6 4 | 4496.8 3 | 4641.8 1 |
| 6 | 4575.7 3 | 4944.1 3 | 5066.9 3 | 4645.4 2 | 4795.5 5 |
| 5 | 4677.5 7 | 5055.0 4 | 5180.8 6 | 4749.7 3 | 4903.4 7 |
| 4 | 4743.8 2 | 5127.1 7 | 5254.9 5 | 4817.5 6 | 4973.6 5 |
| 3 | 4782.0 8 | 5168.8 3 | 5297.7 5 | 4856.7 4 | 5014.1 9 |
| 2 | 4800 4 | 5188.3 4 | 5317.7 9 | 4875.0 9 | 5033.1 7 |
| 1 | 4805.2 6 | 5194.1 1 | 5323.7 2 | 4880.5 2 | 5038.8 |

Table 4: Storey Shears for "E" shaped model

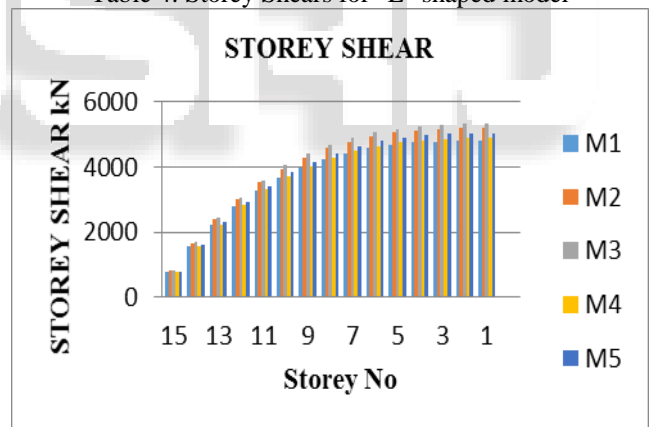


Fig. 13: Variation of Storey Shear for model 1-5

The above Table 4 shows the storey shear for E shaped model 1 to model 5. Storey shear goes on decreases as the storey high increases. It is seen that storey shear for model 4 is less on top storey compared to other models. The Fig 13 shows the graphical representation of storey shear for model 1 to model 5.

| STORE Y NO | M6 (kN) | M7 (kN) | M8 (kN) | M9 (kN) | M10 (kN) |
|------------|-------------|-------------|-------------|-------------|-------------|
| 15 | 876.61 | 905.05 | 931.43 | 874.05 | 890.11 |
| 14 | 1713.4 4 | 1819.0 6 | 1881.6 | 1757.7 4 | 1800.9 7 |
| 13 | 2436.3 | 2608.5 9 | 2702.3 6 | 2521.0 7 | 2587.7 7 |
| 12 | 3053.5 3 | 3282.7 5 | 3403.1 9 | 3172.8 5 | 3259.6 |

| | | | | | |
|----|-------------|-------------|-------------|-------------|-------------|
| 11 | 3573.4 6 | 3850.6 4 | 3993.5 4 | 3721.9 | 3825.5 3 |
| 10 | 4004.4 5 | 4321.3 8 | 4482.9 | 4177.0 1 | 4294.6 3 |
| 9 | 4354.8 2 | 4704.0 6 | 4880.7 2 | 4547 | 4676 |
| 8 | 4632.9 1 | 5007.8 1 | 5196.4 8 | 4840.6 7 | 4978.6 9 |
| 7 | 4847.0 7 | 5241.7 2 | 5439.6 4 | 5066.8 2 | 5211.7 9 |
| 6 | 5005.6 3 | 5414.9 1 | 5619.6 8 | 5234.2 6 | 5384.3 8 |
| 5 | 5116.9 4 | 5536.4 8 | 5746.0 6 | 5351.8 | 5505.5 4 |
| 4 | 5189.3 3 | 5615.5 5 | 5828.2 6 | 5428.2 4 | 5584.3 3 |
| 3 | 5231.1 4 | 5661.2 2 | 5875.7 3 | 5472.3 9 | 5629.8 4 |
| 2 | 5250.7 2 | 5682.6 | 5897.9 6 | 5493.0 6 | 5651.1 5 |
| 1 | 5256.4 6 | 5688.9 2 | 5904.5 4 | 5499.1 8 | 5657.4 7 |

Table 5: Storey Shears for “Diaphragm Discontinuity” model

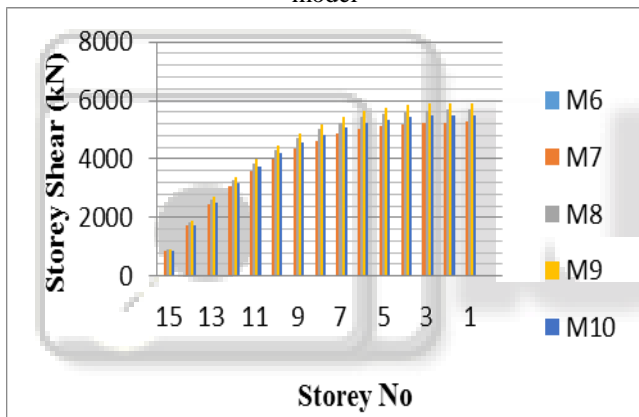


Fig. 14: Variation of Story Shear for model 6 – 10

The above Table 5 shows the point displacement for diaphragm discontinuity model 6 to model 10. Storey shear goes on decreases as the storey height increases. It is seen that storey shear for model 9 is less on top storey compared to other models. The Fig 14 shows variation of storey shear for model 6 to model 10.

C. Storey Drifts

| STORE Y NO | M1 (m) | M2 (m) | M3 (m) | M4 (m) | M5 (m) |
|------------|-------------|-------------|-------------|-------------|-------------|
| 15 | 0.0017 2 | 0.0013 0 | 0.0019 2 | 0.0010 6 | 0.0019 4 |
| 14 | 0.0022 5 | 0.0013 7 | 0.0019 4 | 0.0011 2 | 0.0019 6 |
| 13 | 0.0027 8 | 0.0014 2 | 0.0019 6 | 0.0011 7 | 0.0019 7 |
| 12 | 0.0032 6 | 0.0015 0 | 0.0019 5 | 0.0012 5 | 0.0019 5 |
| 11 | 0.0036 7 | 0.0015 0 | 0.0019 5 | 0.0012 5 | 0.0019 5 |

| | | | | | |
|----|-------------|-------------|-------------|-------------|-------------|
| 10 | 0.0039 9 | 0.0015 2 | 0.0019 2 | 0.0012 7 | 0.0019 2 |
| 9 | 0.0042 4 | 0.0015 2 | 0.0018 6 | 0.0012 8 | 0.0018 6 |
| 8 | 0.0044 2 | 0.0014 9 | 0.0017 8 | 0.0012 7 | 0.0017 8 |
| 7 | 0.0045 4 | 0.0014 4 | 0.0016 8 | 0.0012 3 | 0.0016 6 |
| 6 | 0.0046 1 | 0.0013 6 | 0.0015 4 | 0.0011 7 | 0.0015 2 |
| 5 | 0.0046 1 | 0.0012 5 | 0.0013 6 | 0.0010 8 | 0.0013 5 |
| 4 | 0.0045 5 | 0.0011 0 | 0.0011 5 | 0.0009 5 | 0.0011 4 |
| 3 | 0.0043 8 | 0.0009 1 | 0.0009 1 | 0.0007 9 | 0.0008 9 |
| 2 | 0.0039 3 | 0.0006 7 | 0.0006 2 | 0.0005 9 | 0.0006 0 |
| 1 | 0.0023 8 | 0.0003 2 | 0.0002 6 | 0.0003 | 0.0002 5 |

Table 6: Storey Drift for "E" shaped model

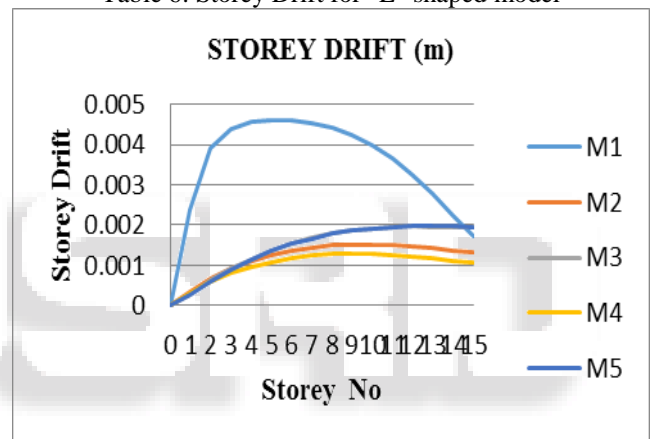


Fig. 15: Variation of Storey Drift for Model 1 to Model 5

The above Table 6 shows the storey drift for E shaped model 1 to model 5. Storey drift goes on increases as the storey height increases. It is seen that storey drift for model 5 is more on top storey compared to other models. The Fig-15 indicate the graphical representation of story drift for model 1 to model 5.

| STORE Y NO | M6 (m) | M7 (m) | M8 (m) | M9 (m) | M10 (m) |
|------------|-------------|-------------|-------------|-------------|-------------|
| 15 | 0.0010 8 | 0.0018 1 | 0.0001 6 | 0.0014 7 | 0.0001 5 |
| 14 | 0.0016 1 | 0.0018 5 | 0.0001 6 | 0.0015 0 | 0.0001 6 |
| 13 | 0.0021 3 | 0.0018 7 | 0.0001 7 | 0.0015 3 | 0.0001 6 |
| 12 | 0.0026 0 | 0.0018 8 | 0.0001 7 | 0.0015 5 | 0.0001 7 |
| 11 | 0.0030 0 | 0.0018 8 | 0.0001 8 | 0.0015 6 | 0.0001 7 |
| 10 | 0.0033 3 | 0.0018 6 | 0.0001 8 | 0.0015 5 | 0.0001 7 |
| 9 | 0.0035 9 | 0.0018 2 | 0.0001 7 | 0.0015 2 | 0.0001 7 |

| | | | | | |
|---|-------------|-------------|-------------|-------------|-------------|
| 8 | 0.0037 9 | 0.0017 5 | 0.0001 7 | 0.0014 7 | 0.0001 6 |
| 7 | 0.0039 4 | 0.0016 5 | 0.0001 6 | 0.0014 0 | 0.0001 5 |
| 6 | 0.0040 5 | 0.0015 2 | 0.0001 5 | 0.0013 0 | 0.0001 4 |
| 5 | 0.0041 1 | 0.0013 6 | 0.0001 4 | 0.0011 7 | 0.0001 3 |
| 4 | 0.0041 2 | 0.0011 6 | 0.0001 2 | 0.0010 0 | 0.0001 2 |
| 3 | 0.0040 6 | 0.0009 3 | 0.0001 0 | 0.0008 1 | 0.0001 0 |
| 2 | 0.0037 9 | 0.0006 5 | 0.0000 8 | 0.0005 8 | 0.0000 8 |
| 1 | 0.0023 8 | 0.0003 0 | 0.0000 5 | 0.0002 7 | 0.0000 5 |

Table 7: Story Drifts for "Diaphragm Discontinuity" model

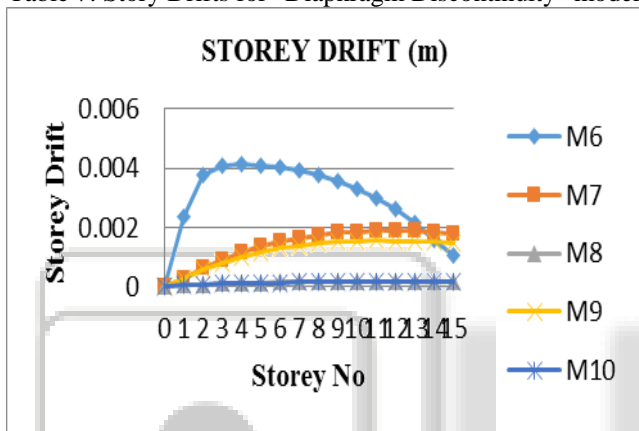


Fig. 16: Variation of Storey Drift for Model 6 to Model 10

The above Table 7 shows the storey drift for diaphragm discontinuity model 6 to model 10. Storey drift energies on decreases as the storey height increases. It is seen that storey drift for model 10 is less on top storey compared to other models. The Fig 16 shows the difference of storey drift for model 6 to model 10.

D. Point Displacement

| STORE Y NO | M1 (m) | M2 (m) | M3 (m) | M4 (m) | M5 (m) |
|------------|------------|------------|-------------|-------------|--------|
| 15 | 0.156 7 | 0.048 7 | 0.0379 | 0.0426 | 0.0352 |
| 14 | 0.150 5 | 0.045 2 | 0.0348 | 0.0397 | 0.0322 |
| 13 | 0.144 8 | 0.041 7 | 0.0315 | 0.0366 | 0.0292 |
| 12 | 0.137 4 | 0.037 9 | 0.0283 | 0.0333 | 0.0262 |
| 11 | 0.128 7 | 0.034 1 | 0.0251 1 | 0.03 | 0.0232 |
| 10 | 0.118 7 | 0.030 1 | 0.0219 | 0.0266 | 0.0202 |
| 9 | 0.107 8 | 0.026 2 | 0.0187 | 0.0231 9 | 0.0173 |
| 8 | 0.096 1 | 0.022 3 | 0.0156 4 | 0.0197 | 0.0144 |

| | | | | | |
|---|------------|------------|-------------|-------------|-------------|
| 7 | 0.083 8 | 0.018 4 | 0.0126 | 0.0163 | 0.0116 |
| 6 | 0.071 2 | 0.014 7 | 0.0099 | 0.0131 | 0.0091 |
| 5 | 0.058 2 | 0.011 2 | 0.0073 | 0.0100 4 | 0.0067 |
| 4 | 0.045 | 0.008 | 0.0051 | 0.0072 | 0.0046 |
| 3 | 0.032 3 | 0.005 2 | 0.0031 | 0.0047 5 | 0.0028 |
| 2 | 0.019 6 | 0.002 9 | 0.0016 2 | 0.0026 5 | 0.0014 |
| 1 | 0.008 1 | 0.001 1 | 0.0005 5 | 0.0010 0 | 0.0005 1 |

Table 8: Point Displacement for "E" shaped model

The above Table 8 shows the point displacement for E shaped model 1 to model 5. Point displacement goes on increases as the storey height increases. It is seen that point displacement for model 1 is more on top storey compared to other models. The Fig 17 shows the variation of for model 1 to model 5.

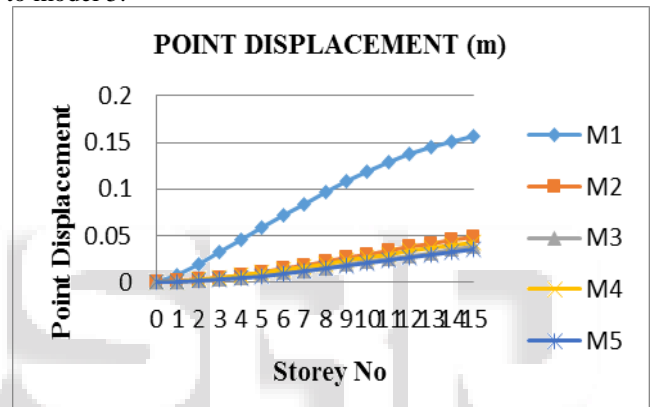


Fig. 17: Point Displacement for Model 1 to Model 5

| STORE Y NO | M6 (m) | M7 (m) | M8 (m) | M9 (m) | M10 (m) |
|------------|-------------|------------|-------------|-------------|-------------|
| 15 | 0.1435 | 0.067 | 0.0066 | 0.0562 | 0.0063 8 |
| 14 | 0.1403 | 0.061 6 | 0.0061 | 0.0518 | 0.0059 3 |
| 13 | 0.1355 | 0.056 | 0.0056 7 | 0.0472 | 0.0054 4 |
| 12 | 0.1291 | 0.050 4 | 0.0051 5 | 0.0426 | 0.0049 4 |
| 11 | 0.1213 | 0.044 | 0.0046 2 | 0.038 | 0.0044 4 |
| 10 | 0.1123 | 0.039 1 | 0.004 | 0.0333 | 0.0039 2 |
| 9 | 0.1023 | 0.033 5 | 0.0035 5 | 0.0287 1 | 0.0034 |
| 8 | 0.0916 | 0.028 1 | 0.0030 2 | 0.0241 | 0.0029 |
| 7 | 0.0802 | 0.022 8 | 0.0025 | 0.0197 2 | 0.0024 |
| 6 | 0.0684 4 | 0.017 9 | 0.0020 1 | 0.0155 2 | 0.0019 |
| 5 | 0.0563 | 0.013 3 | 0.0016 | 0.0116 | 0.0015 |

| | | | | | |
|---|--------|------------|-------------|--------|--------|
| 4 | 0.044 | 0.009 3 | 0.0011 4 | 0.0081 | 0.0011 |
| 3 | 0.0317 | 0.005 8 | 0.0008 | 0.0051 | 0.0007 |
| 2 | 0.0195 | 0.003 | 0.0005 | 0.0027 | 0.0004 |
| 1 | 0.0082 | 0.001 | 0.0002 | 0.0009 | 0.0002 |

Table 9: Point Displacement for "Diaphragm Discontinuity" model

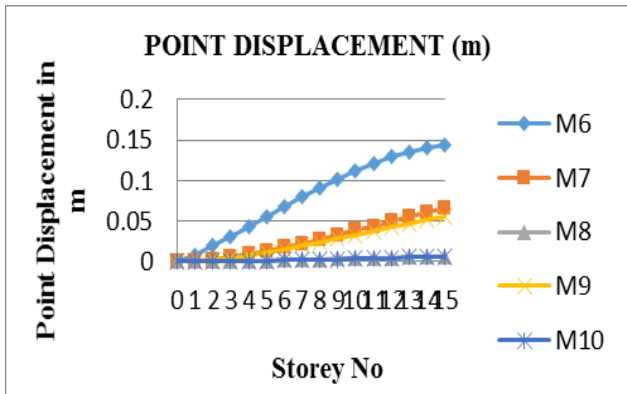


Fig. 18: Point Displacement for model 6 to model 10

The above Table 9 shows the point displacement for diaphragm discontinuity model 6 to model 10. Point displacement goes on increases as the storey height increases. It is understood that point displacement for model 6 is more on top storey compared to other models. The Fig 18 indicate the variation of storey drift for model 6 to model 10.

E. Pushover Curves

1) Pushover curves For E-Shaped (ES) & Diaphragm discontinuity (DD) Model

| STEP | DISPLACEMENT(m) | | BASEFORCE(kN) | |
|------|-----------------|--------|---------------|----------|
| | ES | DD | ES | DD |
| 0 | 0 | 0 | 0 | 0 |
| 1 | 0.0475 | 0.1251 | 1603.065 | 4591.635 |
| 2 | 0.0508 | 0.2442 | 1698.246 | 8481.374 |
| 3 | 0.0539 | 0.2758 | 1734.596 | 8997.638 |
| 4 | 0.0696 | 0.3189 | 1818.701 | 9337.473 |
| 5 | 0.2548 | 0.3698 | 2214.583 | 9542.388 |
| 6 | 0.4439 | 0.3698 | 2489.943 | 9319.982 |
| 7 | 0.5222 | 0.3755 | 2589.558 | 9382.83 |
| 8 | 0.4906 | 0.3755 | 1426.410 | 9322.377 |
| 9 | - | 0.3791 | - | 9380.440 |
| 10 | - | 0.3888 | - | 9468.168 |
| 11 | - | 0.4003 | - | 9506.372 |
| 12 | - | 0.3365 | - | 6911.181 |

Table 10: Pushover table for Bare Frame

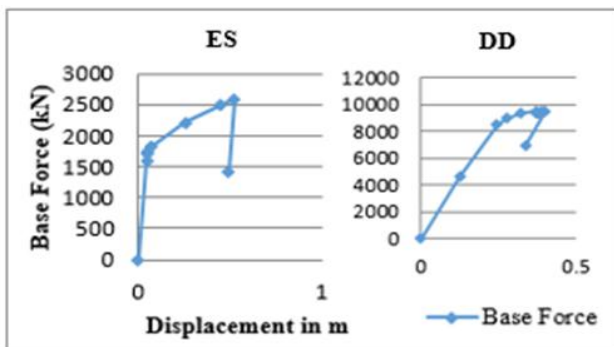


Fig. 19: Pushover curve for Bare Frame

| STEP | DISPLACEMENT(m) | | BASEFORCE(kN) | |
|------|-----------------|--------|---------------|----------|
| | ES | DD | ES | DD |
| 0 | 0 | 0 | 0 | 0 |
| 1 | 0.0068 | 0.0487 | 5220.751 | 4783.962 |
| 2 | 0.0451 | 0.2323 | 33153.54 | 21258.01 |
| 3 | 0.0451 | 0.4154 | 33133.69 | 33546.32 |
| 4 | 0.0511 | 0.4801 | 37178.76 | 37047.35 |
| 5 | 0.0511 | 0.2201 | 37029.01 | 11337.46 |
| 6 | 0.0577 | - | 41218.21 | - |
| 7 | 0.0577 | - | 40992.84 | - |
| 8 | 0.0583 | - | 41403.96 | - |
| 9 | 0.0583 | - | 41254.33 | - |
| 10 | 0.0593 | - | 41827.11 | - |
| 11 | 0.0593 | - | 41739.27 | - |
| 12 | 0.0611 | - | 42851.48 | - |
| 13 | 0.0611 | - | 42754.58 | - |
| 14 | 0.0627 | - | 43741.92 | - |
| 15 | 0.0627 | - | 43658.98 | - |
| 16 | 0.0651 | - | 45109.56 | - |

Table 11: Pushover table for SMRF with Shear Wall at re-entrant corner (ES) / corner (DD)

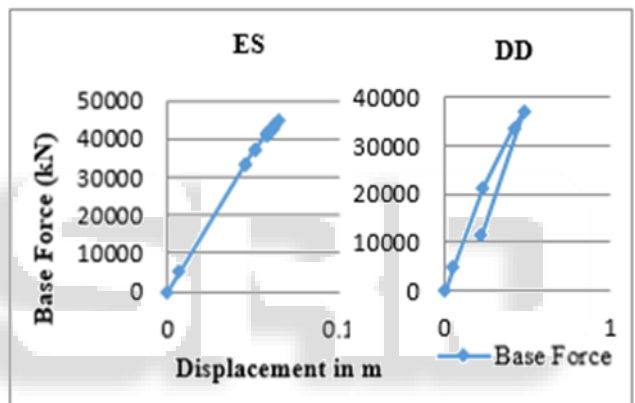


Fig. 21: Pushover curve for SMRF with Shear Wall at re-entrant corner

| STEP | DISPLACEMENT(m) | | BASEFORCE(kN) | |
|------|-----------------|--------|---------------|----------|
| | ES | DD | ES | DD |
| 0 | 0 | 0 | 0 | 0 |
| 1 | 0.0505 | 0.0181 | 6401.33 | 16257.56 |
| 2 | 0.2368 | 0.2015 | 28707.4 | 160802.5 |
| 3 | 0.4239 | 0.2899 | 47633.6 | 226149.2 |
| 4 | 0.4294 | 0.29 | 48154.4 | 225993.6 |
| 5 | 0.4294 | 0.3155 | 47702.3 | 246379.3 |
| 6 | 0.4747 | 0.2915 | 52178.60 | 224220.9 |
| 7 | 0.4747 | - | 51889.03 | - |
| 8 | 0.4912 | - | 53409.36 | - |
| 9 | 0.4912 | - | 53244.65 | - |
| 10 | 0.5253 | - | 56322.65 | - |
| 11 | 0.5099 | - | 54316.79 | - |

Table 12: Pushover table for SMRF with Shear Wall at Periphery

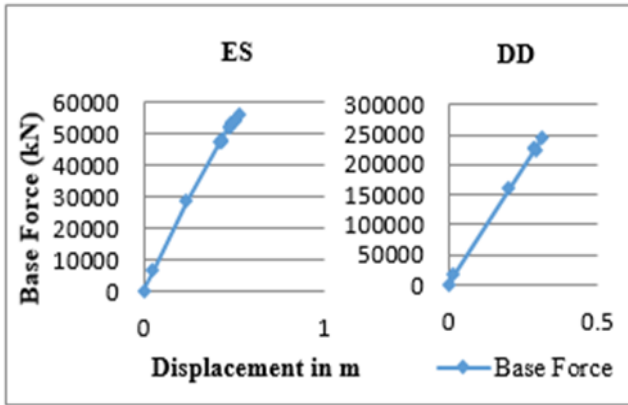


Fig. 23: Pushover curve for SMRF with Shear wall at Periphery (ES)

| STEP | DISPLACEMENT(m) | | BASEFORCE(kN) | |
|------|-----------------|--------|---------------|----------|
| | ES | DD | ES | DD |
| 0 | 0 | 0 | 0 | 0 |
| 1 | 0.0201 | 0.0499 | 2528.226 | 4247.221 |
| 2 | 0.2042 | 0.233 | 15904.85 | 15325.89 |
| 3 | 0.2754 | 0.4248 | 19972.79 | 23092.26 |
| 4 | 0.2754 | 0.5556 | 19900.62 | 28163.10 |
| 5 | 0.2786 | 0.5556 | 20102.83 | 28107.64 |
| 6 | 0.2786 | 0.6435 | 20073.12 | 31932.79 |
| 7 | 0.2789 | 0.6436 | 20089.54 | 31218.32 |
| 8 | - | 0.6837 | - | 32843.97 |
| 9 | - | 0.5826 | - | 24556.62 |

Table 13: Pushover table for Flat Slab with Shear Wall at re-entrant corner (ES) / corner (DD)

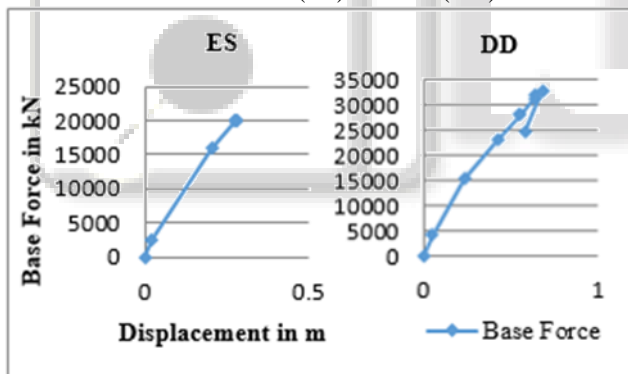


Fig. 25: Pushover curve for Flat Slab with Shear Wall at re-entrant corner/corner

| STEP | DISPLACEMENT(m) | | BASEFORCE(kN) | |
|------|-----------------|--------|---------------|----------|
| | ES | DD | ES | DD |
| 0 | 0 | 0 | 0 | 0 |
| 1 | 0.0071 | 0.0129 | 5338.762 | 11552.92 |
| 2 | 0.0401 | 0.1953 | 28348.52 | 147342.5 |
| 3 | 0.039 | 0.2713 | 27400.13 | 199444.1 |
| 4 | - | 0.2713 | - | 199304.3 |
| 5 | - | 0.2736 | - | 200972.6 |
| 6 | - | 0.2116 | - | 145484.1 |

Table 14: Pushover table for Flat Slab with Shear Wall at Periphery

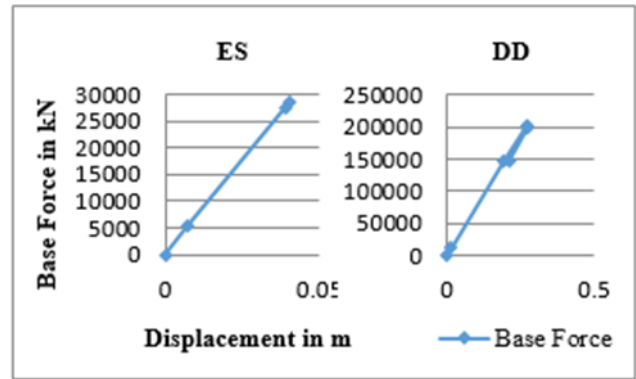


Fig. 27: Pushover curve for Flat Slab with Shear Wall at Periphery

V. CONCLUSIONS

- 1) In E shaped model the base shears and storey shears is found to be highest for SMRF with shear wall at periphery dual system when compared to all the other E shaped models.
- 2) Storey drifts obtained in E model for SMRF with shear wall at periphery dual system is far better compared to other models and 58.5% better than bare frame model.
- 3) 3 Point displacements obtained for E model illustrated better results for SMRF with shear walls at periphery dual system compared to other variants and about 62.29 % better than bare frame model.
- 4) From the graphs plotted it is clearly seen that the bare frame for E-Shaped & Diaphragm discontinuity model is the most vulnerable model in the seismic zone V owing to the absence of lateral load resisting system.
- 5) In Diaphragm discontinuity models, the base shears and storey shears indicated higher values for the model, flat slabs with shear walls at periphery, compared to other diaphragm discontinuity models.
- 6) Point displacements obtained for models with diaphragm discontinuity having, SMRF with shear wall at periphery and flat slab with shear wall at periphery show closely matching results with the previous model showing better result than the latter one and 90% better than bare frame model.
- 7) When both E-Shape and Diaphragm discontinuity models are compared, the graphs illustrated the Diaphragm discontinuity model with shear wall at periphery and flat slabs with shear walls at periphery showed a better performance for storey drift and point displacements compared to all other variants. Thus making it a better dual system than all the other systems considered in this study.

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